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Spatial Deterministic Modelling of Landslide in Chamoli Area

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ABSTRACT

The high susceptibility to landslides of the Himalayan terrain is mainly due to a complex geological setting combined with contemporary crustal movement, varying slopes and relief, heavy rainfall, along with increasing human interference of human in ecosystem. The paper presents Remote Sensing and GIS based spatial data analysis for landslide mapping in Chamoli district using IRS 1C LISS III, IRS 1C PAN image. Landslide map were prepared to understand the parameter which is influencing landslides. It includes the derivation of input parameters from remotely sensed data with limited field check. Factor of safety map is also calculated for the study area.

Keywords: Landslide, GIS, DEM, Factor of Safety

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1. Introduction

The Indian subcontinent has a highly diversified range of natural features. The entire Himalayan range is tectonically sensitive for earthquake and the state Uttarakhand is not an exception (Naithani et al., 2008). The Himalayas, which are the young fold mountain and where the phenomena of stress release is very common together with the uncertain monsoon winds make the region highly prone to natural disasters. The region being the most populous in the world further adds to the damage caused by the natural disasters. This region is prone mainly to earthquakes, landslides, avalanches and forest fire. Landslide may be defined as, "the movement of mass of rock, debris, or earth down a slope (Cruden, 1991). Landslides are a type of 'mass wasting' which denotes any down slope movement of soil and rock under the direct influence of gravity. The term "landslide" encompasses events such as rock falls, topples, slides, spreads and flows (Cruden & Varnes, 1996).

Rainfall, earthquakes, volcanic activity, and changes in groundwater, disturbance and change of a slope by man-made construction activities, or any combination of these factors can initiate landslides. Landslide occurs in the hilly terrain. The Himalaya being geologically young and susceptible to earthquakes



makes it susceptible for landslide. Intensive soil erosion is also one of the main factors for the occurrences of landslides. Over the years, due to increasing cultural activity, the incidences of landslides have shown a disturbing trend of occurrence with higher damage to life and property.

Landslides cause approximately 1000 deaths per year, causing property damage of approximately US \$4 billion (Alexander, 1995). Landslides pose serious threats to settlements, and structures that support transportation, natural resources management and tourism. They cause considerable damage to highways, railways, waterways and pipelines. They commonly occur with other major natural disasters such as earthquakes (Keefer, 1984), volcanic activity (Kimura and Yamaguchi, 2000), and floods caused by heavy rainfall. Landslide occurs in various geological environments, ranging from steep rock slopes to gentle slopes with unconsolidated sediments. In many cases, expanded development and human activities, such as modified slope and deforestation can also increase the incidence of landslide disasters. The increasing trend of landslide occurrences will continue in future with the increased unplanned urbanization and development, continued deforestation and increased regional precipitation due to unprecedented rainfall may be because of changing climatic patterns (Naithani et al., 2013). The landslides occur mostly during the monsoon season when huge precipitation saturates the unstable soil, leading to high erosion rates, and high likelihoods of landslides (Bhasin et al., 2002). Landslide can be rapid or slow, and occur in a wide variety of geologic environments, including underwater. Other effects are upstream and downstream flooding due to landslide dams and dam breaks. (Evans and Savigny, 1994).

In Himalayas natural hazard such as landslide causes havoc to both property and life every year. Landslide hazard Zonation of areas affected by landslides therefore is essential for future development planning and organization of various disasters and mitigation programs.

Aerial photographs were used by Indian Institute of Remote Sensing (IIRS) to study slope failure processes in Alaknanda valley. Large scale aerial photographs on 1:10,000 (approx.) scale supplemented by limited use of Landsat MSS data were analysed for evaluating slope conditions in Tapoban-Vishnugad and Bowala-Nandprayag Hydel scheme in Chamoli district of Uttaranchal (Lakhera, 1982). Remote sensing data of IRS 1A LISS II were integrated in GIS environment by Lakhera et al., (1992) to prepare the landslide hazard zone map in parts of Chamoli region. The major objectives of the study are preparation of the landslide map and to understand the parameter which is influencing landslides. It includes the derivation of input parameters from remotely sensed data with limited field check. Factor of safety map is calculated for the study area.

2. Study area

Chamoli is surrounded by the district of Uttarkashi in the NW, Pithoragarh in the NE, Almora in the SE. the total geographic area is around 7520 sq. km. Gopeshwar is the district headquarters. The nearest airport is Jollygrant and nearest railway station is at Rishikesh. The study area is bounded by North 30° 22' 30'' to 30° 30' 00'' and East 79° 15' 00'' to 79° 22' 30'' in survey of India toposheet no. 53 N/7 NW covering village like Bhimtalla, Chamoli town, Gopeshwar and Mandal.

3. Methodology

The research work is carried out using IRS 1C LISS III, IRS 1C PAN image in addition with SRTM DEM. SOI Toposheet 53 N/7 NW on 1:25,000 is also used for the study.

In order to achieve the objectives of the study, the following methodology has adopted for the entire work. The whole work of the study has been carried out in three stages.

- 1. Pre-Field (Mainly literature survey)
- 2. Field work (Data Collection)
- 3. Post Field (Mainly analysis part and modelling)

3.1 Modelling:

Following steps were taken for modelling-

Compute the static factor of safety (ratio of resisting to driving forces)

- i. Using compiled shear-strength data, assign representative shear strengths to each unit on the geologic map, which yield friction (Φ) and cohesion (c') grids.
- ii. Construct a slope map from digital elevation model (DEM).

iii. Combine shear-strength and slope data in factor of safety equation to estimate static factors of safety in each grid cell.

3.2. Selection of Strength Parameter

A critical aspect of creating a representative slope stability model is the selection of strength parameters. Ultimately, this data will have to be collected on a site-specific basis. Distribution of the material (Lithology) is shown, on 1:25,000 scale that are based on the geological mapping of the region description for each unit are based on field observation, image interpretation, published reports geological/geo-morphological maps, geotechnical strength parameters such as cohesion value and friction angle have been taken for lithosphere assigned to the samples taken from the field observation.

3.3. Data Available for modelling

- i. Comprehensive inventory of triggered landslides.
- ii. Geological map of study area.
- iii. Engineering properties of geologic units.
- iv. DEM of the topography.

3.4. Computing the Static Factor of Safety

The dynamic stability of a slope is related to its static stability; therefore, the static factor of safety for each grid cell must be determined. For purpose of regional analysis, we use a relatively simple limit-equilibrium model of an infinite slope in a material having both frictional and cohesive strength. The factor of safety (Fs) in these conditions is given by:

 $FS= (c'/yt Sin\alpha) + (tan \Phi'/ tan\alpha) - (my_w tan \Phi /y tan\alpha) \dots (1)$

Where:

 Φ ' is the effective friction angle c' is the effective cohesion α is the slope angle y is the material unit weight y_w is the unit weight of water

t is the slope-normal thickness of the failure slab m is the proportion of the slab thickness that is saturated

The equation is written so that the first term on the right side accounts for the cohesive component of the strength, the second term accounts for the frictional component, and the third term accounts for the reduction in frictional strength due to pore pressure. In the conditions modelled for this calibration, no pore-water pressure is included, thus m=0. Thus the third term drops from the equation. Then the factor of safety is then calculated, by inserting values from, cohesion and slope angle grids into equation (1).

4. Analysis

The geology map of the study area is modified from the existing geological maps. The thrust is marked by with the help of Valdiya's map. The LISS-III and PAN merged satellite image was also helpful in the demarcation of the litho-units. The landslide map was prepared from hard copy as well as from the soft copy by applying the image enhancement technique. First of all the Geological map of the study area and cohesion, friction angle and unit weight for all the rock type which are encountered in the field work were calculated and all the map have been prepared in the Arc Info software. Slope map has been prepared by the DEM of the study area. All the maps have been rasterized in Arc Info software in 50 meter grid size. Using algorithm in the raster calculator in Arc Info software, final map has been prepared. The geotechnical parameters for different rocks are depicted in table no.1.

4.1. Static Factor of Safety

This is the ratio between the forces that make a slope fails to the force that prevent a slope from failing. Using the infinite slope model, the static factor of safety of a slope (FS) can be expressed as:

FS =	C,	Tan Φ '	$m\gamma_w Tan \Phi^{ i}$
	γ d Sin α	- — — — — — — — — — — — — — — — — — — —	γ Tan α

Where: C' effective cohesion, Φ ' effective angle of internal friction, γ material unit weight γ_w unit weight of water α angle of the slope from the horizontal, d normal

 α angle of the slope from the horizontal, d normal depth to the failure surface, and m ratio of the height of the water table above the failure surface to d.

The equation is written so that the first term on the right side accounts for the cohesive component of the strength, the second term accounts for the frictional component, and the third term accounts for the reduction in frictional strength due to pore pressure. In the conditions modelled for this calibration, no pore pressure is included (m=0), because in this case it was supposed that there is dry condition.

5. Conclusion

IRS-LISS-III and PAN data products could provide surface changes related to landslides using various change detection technique. The detailed geologic field investigation is required to calculate hazard zone to carry out such study. The factor of safety mostly depends upon the geotechnical parameters. At many places, unconsolidated sediments are not considered due to lack of information on their thickness, which should be incorporated as these are very vulnerable. Ortho-rectification is highly essential for the rugged topography like in this case. Most of the region of landslide map falls on the areas which have less factor of safety. The area is more prone for the failure of the mass. The area which falls in FS<1 is most unstable area, FS>1 is mod. Unstable and FS<6 is most stable.

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Tables and Figures Used

Rock Type	Cohesion	Friction	Unit Weight
Quartzite	50	32	27
Gneiss	40	30	23
HGM	55	34	26
Phyllite	30	29	24
Qtzschist	31	28	22
Mylonite	35	31	21
Fluvioglacial Material	20	18	20
Alluvium	18	15	18

Table1 Geotechnical Parameters for different rocks

Fig 1 Location Map and satellite Image of the study area



Fig 2 Flow Chart of the Methodology





252 Bhatt, Ray, Naithani & Singh (2014)



Fig 7 Geotechnical Parameters Maps (Cohesion Map, Unit Weight and Friction Angle Map)

Fig 8. Factor of Safety Map

Fig 9. Factor of Safety Map Overlaid with Landslide Map



Factor of Safety Map overlaid with Landslide

